



Desktop Geotechnical Study

4063 N Webster Ave

Perris, California

October 20, 2023

Prepared for:

Chevron
Attention: Ms. Alexandra Wines
6001 Bollinger Canyon Road, D1102
San Ramon, CA 94583


Prepared by:

Stantec Consulting Services Inc.



DESKTOP GEOTECHNICAL STUDY

This document entitled Desktop Geotechnical Study was prepared by Stantec Consulting Services Inc. ("Stantec") for the account of Chevron (the "Client").

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APPENDICES

Appendix A – Site Plan



1.0 INTRODUCTION

Stantec Consulting Services Inc. (Stantec) has prepared this letter report for the Desktop Geotechnical Study for the proposed Chevron Hydrogen Fueling Station site located at 4063 N Webster Ave Perris, California (the Site; **Figures 1 and 2**). We understand that Chevron proposes to establish a hydrogen fueling station on the Site.

1.1 PROPOSED DEVELOPMENT

The hydrogen fueling station will include several hydrogen, diesel, and compressed natural gas (CNG) fuel dispensers, fuel storage buildings, vehicle/truck driveway and parking areas, and landscape areas located just north of an existing Chevron gas station. A layout of the site is attached in Appendix A.

1.2 PURPOSE AND SCOPE OF WORK

1.2.1 Purpose

The purpose of this report is to evaluate publicly available information for the Site and provide an overview of potential geotechnical hazards associated with the proposed project. This report has been prepared in general accordance with accepted geotechnical engineering principles and in general conformance with the approved proposal.

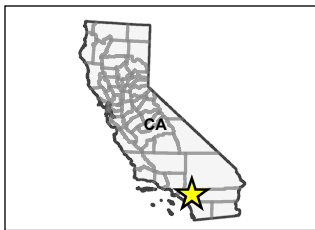
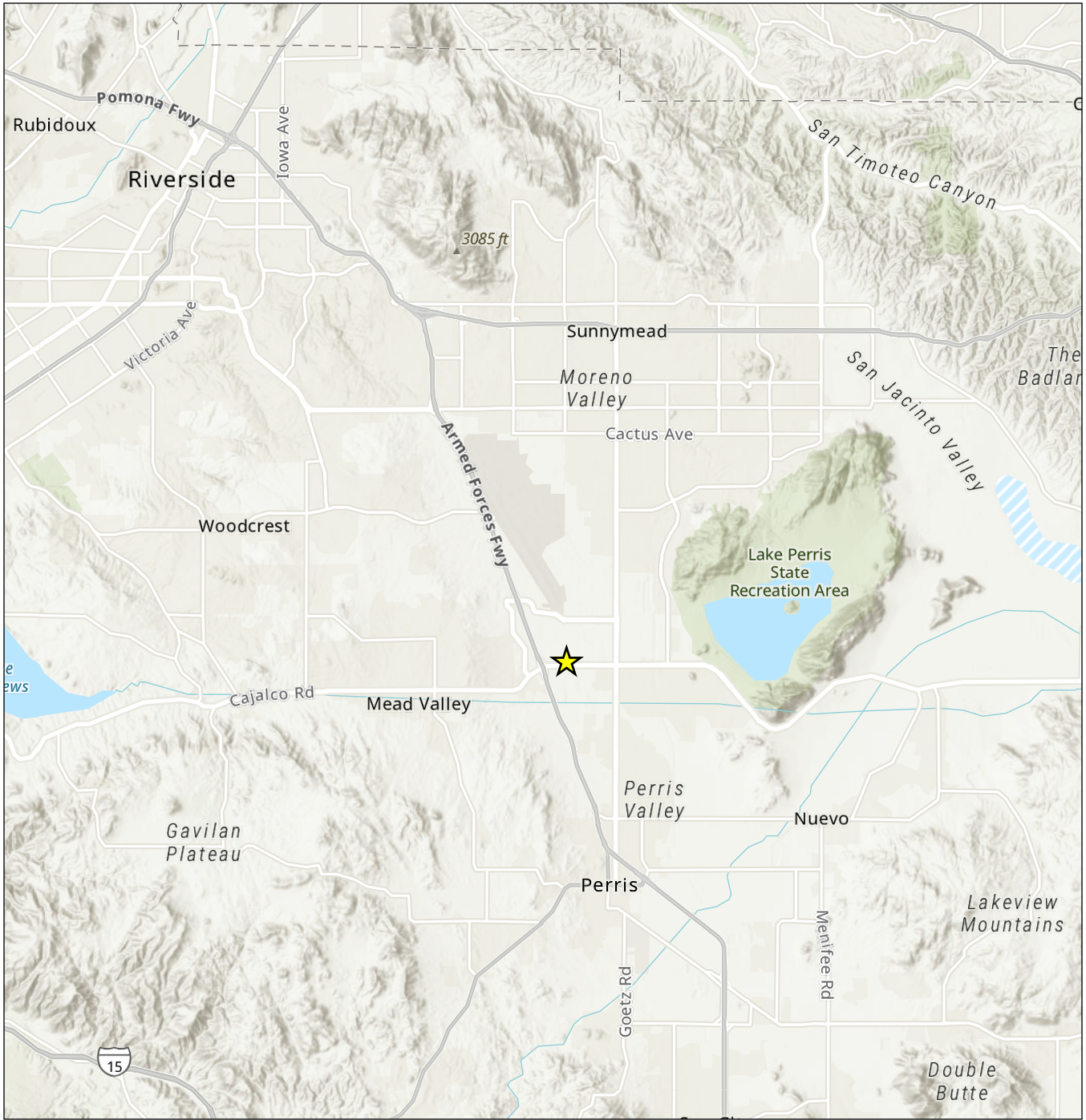
1.2.2 Scope of Work

Our scope of work performed for this project consisted of a review of publicly available data to address the following:

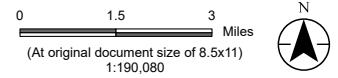
- a) Summary of existing soil conditions from existing geotechnical reports, agricultural, and geologic maps (if available),
- b) Near-surface obstructions or restrictive soil layers (hardpans, rocky soils, etc.) that could impede removal and re-compaction,
- c) Soils that are potentially corrosive to concrete and unprotected steel,
- d) Potentially expansive soils that could impact steel post foundations, equipment pads, shallow building foundations, and concrete slab-on-grade,
- e) Depth to groundwater and potential for ponding water,
- f) Landforms that could develop sinkholes or other hazards,
- g) Frost depth,
- h) Undulating or high-relief terrain,
- i) Onsite faults,
- j) Review of aerial photography to screen for obvious conditions that could impact construction including rock outcrops, road cuts, or other visible indications of difficult soil conditions.



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Legend
★ Project Location



Project Location
Riverside Co., CA
4063 N Webster Ave. Perris, CA 92571

Prepared by PT on 2023-10-18
TR by AS on 2023-10-20
IR by JF on 2023-10-23

Client/Project
Chevron
Chevron Perris H2 Fueling Station

2057297530



Figure No.
1

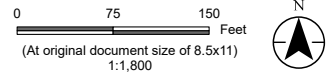
Title
Site Location

- Notes**
- 1. Coordinate System: NAD 1983 UTM Zone 11N
 - 2. Data Sources: Chevron, Stantec, Esri
 - 3. Background: Esri World Imagery

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Legend
 Project Boundary
 Building Parcels



Project Location
Riverside Co., CA
4063 N Webster Ave. Perris, CA 92571

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Client/Project
Chevron
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2057297530

Figure No.
2

Title
Site Boundary

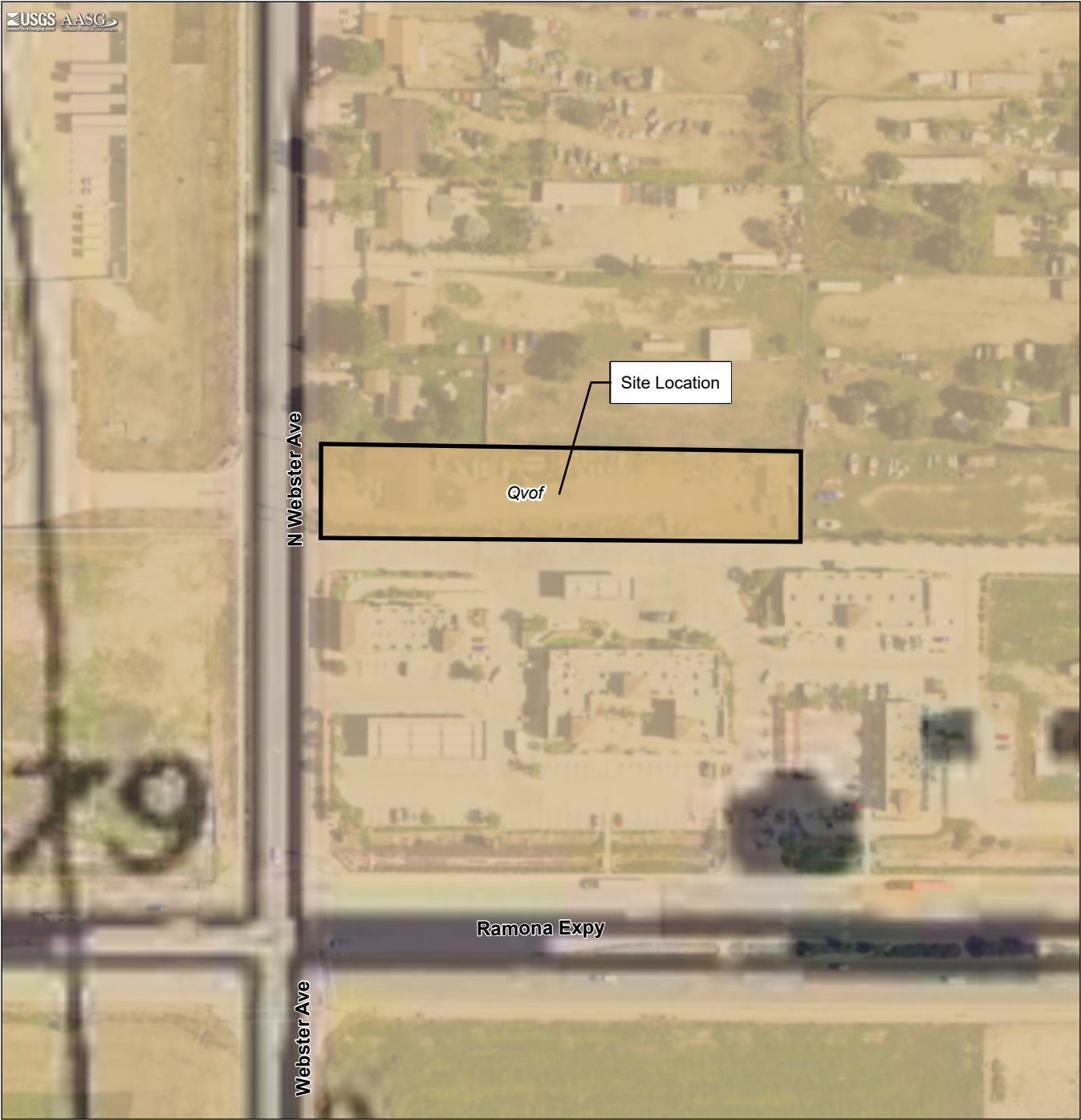
Notes
1. Coordinate System: NAD 1983 UTM Zone 11N
2. Data Sources: Chevron, Stantec, Esri
3. Background: 2021 NAIP

2.0 RESULTS

2.1 REGIONAL GEOLOGY

The Site is located on the Perris block within the northern Peninsular Ranges geomorphic province in Riverside County, California. The geomorphic province is characterized by northwest-trending mountain ranges and valleys which are subparallel to the regional fault systems. San Jacinto strike-slip system is part of the major active fault system approximately 9.5 miles to the east of the site, at the base of the San Jacinto mountains. Another northwest-trending strike-slip fault system, Elsinore fault system, is located approximately 16 miles southwest of the site. The fault system is located at the base of the Santa Anna Mountains. Mountain ranges in the area are comprised of Paleozoic metamorphic complexes, Cretaceous granitic rock, and overlying alluvial material (CGS, 2002). Surficial material at the Site and majority of the developed Perris valley primarily consists of Quaternary alluvium (**Figure 3**).





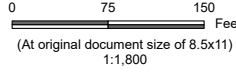


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Legend

-  Project Boundary
- Geologic Units**
-  Qvof: Very old alluvial fan deposits (Pleistocene)



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Client/Project
 Chevron
 Chevron Perris H2 Fueling Station

2057297530

Figure No.
3

Title
Site Geology

- Notes**
1. Coordinate System: NAD 1983 UTM Zone 11N
 2. Data Sources: Chevron, Stantec, Esri, USGS
 3. Background: 2021 NAIP

Disclaimer: This document has been prepared based on information provided by others as cited in the Notes section. Stantec has not verified the accuracy and/or completeness of this information and shall not be responsible for any errors or omissions which may be incorporated herein as a result. Stantec assumes no responsibility for data supplied in electronic format, and the recipient accepts full responsibility for verifying the accuracy and completeness of the data.

2.2 REGIONAL GROUNDWATER

The San Jacinto Basin underlies Perris, Moreno, San Jacinto and Menifee Valleys (DWR, 2023). The California Department of Water Resources periodically monitors groundwater levels in water wells surrounding the Site (DWR, 2023). There are three periodically monitored municipality irrigation wells, two actively monitored state registered observation wells, and one actively monitored USGS well. Casing depth of the state registered observation wells are deeper than 225 feet which may not be representative of the shallower groundwater conditions, while the casing depths for irrigation and USGS wells are unknown. The groundwater level at an irrigation well (Site Code Name 338464N1172319W001) located approximately 0.55 miles east of the Site is reported as 55.9 feet below ground surface (ft bgs) on November 30, 2020. The groundwater level at an irrigation well (Site Code Name 338553N1172491W001) located approximately 0.7 miles north of the Site is reported as 61.8 ft bgs on March 13, 2023. The groundwater level at an irrigation well (Site Code Name 338570N1172480W001) located approximately 0.77 miles east of the Site is reported as 72.4 ft bgs on April 19, 2019. The groundwater level at an observation well (State Well Number 04S03W06Q004S) located approximately 0.55 miles east of the Site is reported as 41.0 ft bgs on April 11, 2023. The groundwater level at an observation well (State Well Number 04S03W06C003S) located approximately 0.76 miles north-east of the Site is reported as 110.2 ft bgs on April 11, 2023. The overall trend of this observation well seems to fluctuate by over 100 feet in the four readings provided which shows reduced confidence in this data point. The groundwater level at an observation well (USGS Site Code Name 335133117140501) located approximately 1 mile north-east of the Site is reported as 30.6 ft bgs on September 19, 2023.

There is an overall increasing groundwater elevations trend over time in the analyzed monitoring and irrigation wells for this part San Jacinto Water Basin. Based on this data, the expected depth to groundwater at the Site is approximately 30 to 72.4 ft bgs.

2.3 REGIONAL SEISMICITY

The Site is located within an area where earthquakes have occurred in recent years. In 1899, a magnitude 6.7 earthquake was recorded approximately 14 miles southeast of the Site in San Jacinto Valley. Two more earthquakes were reported in San Jacinto Valley in 1918, 14 to 15 miles away from the site: one being magnitude 5.0 and the other 6.7. In 1923, a magnitude 6.2 earthquake was recorded approximately 16 miles north of the Site. In 1910, a series of three earthquakes magnitude 5 to 5.4 magnitude, occurring with 3 weeks of each other, were recorded approximately 13.5 miles southwest of the Site. The two 6.7 events in the San Jacinto Valley, were the largest magnitude earthquakes recorded within a 35-mile radius of the Site by the USGS since 1800, with thousands more earthquakes of magnitude less than 5.0 (USGS, 2023a). The USGS has mapped active Quaternary faults within a 35-mile radius of the Site. The closest of these are part of the San Jacinto strike-slip fault system, most recently active during the late Quaternary and it has a high slip rate of 7.0 to 17.0 mm/year primarily consisting of right-lateral movement (SCEDC, 2023). The surface trace of this fault section is located approximately 9.5 miles northeast of the Site within the San Jacinto Valley (U.S. Geological Survey, 2023). Although the fault system is 9.5 or more miles away from the site, moderate to strong ground shaking conditions may be periodically observed on Site.

2.3.1 Strong Ground Shaking

Based on the closest proximal distance to regional faults and magnitude of earthquakes from those faults, moderate ground shaking may occur at the Site. Information published by the USGS indicates the peak ground acceleration



(PGA) with a 2-percent probability of being exceeded at the Site in 50 years is 0.4 to 0.6 g, where g is the acceleration due to gravity determined in accordance with the U.S. Seismic Hazard Maps web site (USGS, 2023). The intensity of shaking will be classified as moderate to strong on the Mercalli scale (Wald et. al. 1999; Linkimer, 2009). Mitigation of moderate to strong ground shaking, when needed, is typically provided by designing structures in accordance with the latest addition of the California Building Code.

2.3.2 Liquefaction

Liquefaction of saturated sandy soils is generally caused by the sudden decrease in soil shear strength due to vibration. During cyclic shaking typically caused by an earthquake, the soil mass is distorted, and inter-particle stresses are transferred from the soil particles to the pore water. As pore pressure increases, the bearing capacity decreases, and the soil may behave temporarily as a viscous fluid (liquefaction), and consequently loses its capacity to support the structures founded thereon.

Engineering research of soil liquefaction potential (Seed & Idriss, 1982; Seed et. al., 1985) indicates that generally, the following three basic factors must exist concurrently for liquefaction to occur:

- A source of ground shaking such as an earthquake capable of generating soil mass distortions.
- A relatively loose sandy soil fabric exhibiting a potential for volume reduction.
- A relative shallow groundwater table (within approximately 50 feet below ground surface) or completely saturated soil conditions that would allow positive pore pressure generation.

The Site is not within a current Liquefaction Zones established by the California Geological Survey (CGS, 2023a) as the area has not been evaluated for liquefaction and landslide hazards. That said, the Site is currently within an Unevaluated Area for Earthquake Zones of Required Investigation, as mapped by the California Geological Survey (CGS, 2023b).

Groundwater at the Site is expected to be approximately 30 to 72.4 feet bgs based on depth to groundwater within monitoring wells in the Site vicinity (DWR, 2023). Based on the near surface soil conditions, anticipated subsurface soil conditions, deep groundwater, and proximity to nearby active earthquake fault lines, it is our opinion that the potential for seismically induced settlement, including liquefaction, is low to moderate.

2.3.3 Lateral Spreading

Lateral spreading typically occurs as a form of horizontal displacement of relatively flat-lying alluvial material toward an open or “free” face such as an open body of water, channel, or excavation. This movement is generally due to failure along a weak plane and is often associated with liquefaction. As cracks develop within the weakened material, blocks of soil displace laterally toward the open face. Cracking and lateral movement may gradually propagate away from the face as blocks continue to break free.

Due to the low to moderate potential for liquefaction and the lack of open bodies of water in proximity to the Site, the potential for lateral spreading is considered low.



2.4 SUBSIDENCE

Land subsidence is a gradual settling or sudden sinking of the Earth's surface due to several factors. Underground mining operations can lead to surface subsidence as subsurface material settles where material has been removed from below. In areas of karst deposits, groundwater dissolves away the limestone or dolomite creating voids under the Earth's surface. Soil types throughout the state also plays a role in land subsidence since some types expand when wet and contract when dry.

In California, the primary cause of coastal land subsidence is due to groundwater withdrawal from unconsolidated aquifers across the State (USGS, 2023b). Although the Site is located within an area of unconsolidated alluvial material, the USGS has not identified land subsidence in the area surrounding the Site (USGS, 2023b). Therefore, the potential for surface subsidence at the Site is considered low.

2.5 SITE CONDITIONS

2.5.1 Expansive Soil Potential

Expansion and contraction of volume can occur when expansive soils undergo alternating cycles of wetting (swelling) and drying (shrinking). Soils on the Site consist primarily of Greenfield sandy loam (NRCS, 2023). Loam soils are generally not considered expansive soils. The design level geotechnical investigation will determine the expansive potential of the underlying soil at the site and any mitigation measures required.

2.5.2 Corrosive Soil Potential

Sand in direct contact with concrete typically has a lower corrosion potential and clays in direct contact with concrete are more susceptible to corrosion. Corrosive soils can typically be mitigated with corrosion resistant concrete or coating of steel. The design level geotechnical investigation will determine the corrosion potential of the underlying soil at the site and any mitigation measures required. Based on our knowledge of the proposed development, steel should not be in direct contact with soil.

2.5.3 Near-surface Obstructions or Restrictive Soil Layers

The Site is overlain by a concrete pad, small building, metal scrap, and potentially low-lying vegetation requiring clearing prior to development. If surface organics are identified, they will be removed prior to grading the Site. The approximate depth of subsurface organic material will be known once the site is drilled for the design level geotechnical investigation.

2.5.4 Frost Depth

The standard frost depth for construction governed by the California Building Code and United States Department of Commerce (USDC) is zero inches in the Riverside County, California. Proposed minimum foundation depths will be required below the frost depth and will be included in the design level geotechnical investigation report.



2.5.5 Slopes

The site is relatively flat, with a topographic gradient less than 5 percent. Permanent slopes steeper than 5:1 (horizontal to vertical) or higher than 5 feet are not anticipated to be constructed or built upon for the Project. Due to the existing topography and the proposed grading, landslides are not considered a potential hazard for the Site. The stability of slopes during foundation excavation should be verified when design-grading information becomes available.

2.5.6 Erosion

The fine-grained soils underlying the site are potentially susceptible to erosion or the loss of topsoil due to surface water flows. However, the site will primarily be paved and therefore should not experience surface erosion.

Mitigation of soil erosion may include selective grading, establishment of anchoring vegetation, design of runoff control features such as drainage ditches, and construction of erosion control features such as pavements and surface mats. These mitigation options should be addressed in the design-level evaluations for the Project.

2.5.7 Landforms that Could Develop Sinkholes or Other Hazards

Sinkholes and karst topography are known to exist in California (USGS, 2000). However, the Site is underlain by old Quaternary alluvial fan material primarily consisting of sand (USGS, 2003). Therefore, the potential for sinkhole development at the Site is considered low. Land subsidence due to groundwater withdrawal is well documented in California but has not been mapped at the Site (USGS, 2023b). Therefore, the potential for subsidence at the Site is considered low. Existing landforms at the Site are not consistent with landforms that could develop into sinkholes or other geologic hazards but should be addressed in the design level geotechnical investigation when site specific conditions are understood.

2.6 REVIEW OF AERIAL PHOTOGRAPHS

Historical aerial imagery from 2002, 2003, 2004, 2005, 2006, 2009, 2011, 2012, 2013, 2014, 2016, 2018, 2019, 2020, 2021, and 2023 reveal that the adjacent properties were developed for their current use between 2004 and 2006, while any residential activities at the site stopped around 2018 via demolition of one of the properties (Google, Inc., 2023).

3.0 CONCLUSIONS

Based on the data reviewed and referenced in this Desktop Geotechnical Study, the possibility of surface fault rupture is unlikely to exist at the Site, but there is potential for moderate to strong ground shaking. Based on the near surface soil conditions, anticipated subsurface soil conditions, and the high PGA, it is our opinion that the potential for liquefaction-related ground failure, including liquefaction, is moderate to low, but additional liquefaction assessment is recommended in the detailed design. In addition, the potential for lateral spreading occurring on the Site is low.

The depth to groundwater, based on data from wells surrounding the Site, is estimated to be 30 to 72.4 ft bgs. The design level geotechnical investigation will further assess the depth to groundwater below the Site if needed. The



DESKTOP GEOTECHNICAL STUDY

potential for subsidence is considered low due to local geologic conditions. The potential for expansive and corrosive soils will be assessed during the design level geotechnical investigation, and mitigation measures may be required. The design level geotechnical investigation will establish the depth of organic soils on site and recommend minimum foundation depths to mitigate against the potential effects of frost. Because the Site is relatively flat, landslides are not considered a potential hazard for the Site. The development of sinkholes is unlikely but will be further assessed during the design level geotechnical investigation. Although the Site soils may be prone to erosion, planned development will pave most of the Site, reducing erosion risks.



4.0 REFERENCES

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APPENDIX A SITE PLAN





SITE INFORMATION				
SITE HOST	MARWAN ALABBASI			
C-STORE BUILDING SF	3842 SF			
POWER UTILITY PROVIDED	TBD			
FUEL DISPENSING				
	GASOLINE	DIESEL	H2	CNG
EXISTING	0	0	0	0
PROPOSED	0	4	3	2
PARKING				
	REGULAR	ADA		
EXISTING	TBD	TBD		
PROPOSED	TBD	TBD		
HYDROGEN				
HRI POTENTIAL	TBD			
H2 STORAGE CONFIGURATION	1200KG 8W x 6H 1000bar (6.5m LONG)			
# OF STATION MODULES	3			

- COMMENTS:
- UTILITY TRANSFORMER AND MSA LOCATIONS TO BE REVIEWED AND CONFIRMED DURING UTILITY APPLICATION PROCESS.
 - EXISTING DRIVEWAY TO BE WIDENED TO ALLOW FOR TURN RADIUS WITH 53' TRAILERS.
 - HYDROGEN FUELING AREA IS CURRENTLY ARRANGED TO ACCOMMODATE EITHER LIGHT OR HEAVY DUTY VEHICLES.
 - ALL DELIVERY TRUCK PATHS ARE MODELED USING A 40' LONG TRAILER. ALL CNG, H2 AND RENEWABLE DIESEL CUSTOMER TRUCK PATHS ARE MODELED USING A 53' LONG TRAILER.

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MARK	DATE	REVISIONS	INITIAL	MARK	DATE	REVISIONS	INITIAL
△	03/31/23	ADDED DIESEL ELECTRICAL BUILDING	CC	△			
△	02/21/23	ISSUED FOR APPROVAL	BSE	△			
△	12/16/22	ISSUED FOR APPROVAL	LGH	△			
△	11/23/22	ISSUED FOR REVIEW	BSE	△			

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4063 N WEBSTER AVE
 PERRIS, CA 92571
 SERVICE STATION #

SITE PLAN

MILESTONES			
PROJECT PHASE	DATE	INITIALS	SS#
ISSUED FOR PLANNING	--	--	--
ISSUED FOR PERMIT	--	--	--
ISSUED FOR BID	--	--	--
ISSUED FOR CONSTRUCTION	--	--	--

JOB# 2057297530
 SCALE: AS NOTED

SHEET
SP1

ISSUED FOR APPROVAL
 03/31/23

